

FULL-WAVEFORM INVERSION OF SH-WAVE INPUT MOTIONS IN A NEAR-SURFACE 2D DOMAIN

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Abstract. There is a need to estimate complex seismic input motions in a near-surface domain, without resorting to the hypocenter, from restricted seismic measurement data. Thus, engineers can replicate responses within structures and soils after an earthquake occurrence by using the estimated seismic inputs and evaluate the impact of an earthquake on the built environment. To date, there has been no robust numerical method that can identify complex seismic input motions in a solid, truncated by a wave-absorbing boundary condition. Existing methods are limited to large-scale seismic-source inversion approaches and deconvolution. To fill this gap, a new inversion modeling method is presented for reconstructing complex, incoherent SH wave input motions in a two-dimensional (2D) domain that is truncated by a wave-absorbing boundary condition (WABC), using a partial differential equation (PDE)-constrained optimization method. In a set of numerical examples, targeted, dynamic traction at the WABC mimics seismic incident wavefield. The discretize-then-optimize (DTO) approach is used in the mathematical modeling and numerical implementation, and the finite element method (FEM) is applied to solve state and adjoint problems. The numerical results indicate that the presented inversion algorithm can reconstruct incident, inclined plane waves. Second, the accuracy of our inversion solver is not compromised by the material complexity of a background domain. Lastly, the minimizer suffers less from solution multiplicity when it identifies lower frequency traction (e.g., a realistic seismic signal).

1 INTRODUCTION

To date, there has been no robust numerical method that can identify complex seismic input motions in a solid, truncated by a wave-absorbing boundary condition. Existing methods are limited to large-scale seismic-source inversion approaches [1] and deconvolution [14, 29–31]. As an alternative to such conventional methods, this paper presents a new numerical method to identify arbitrary, incoherent incoming seismic waves in a multi-dimensional truncated domain using sparse seismic measurement. To this end, we discretize the spatial and temporal distribution of an estimated incident wave and calculate the discretized parameters of the estimated wave that can minimize the discrepancy between the measured data at sensor locations and their computed counterparts induced by the estimated wave. The PDE-constrained optimization framework has advanced the inverse problems associated with elastodynamic wave motions, and the associated applications span from material and scatterer characterization to dynamic-input identification. For example, the geotechnical site characterization methodologies have

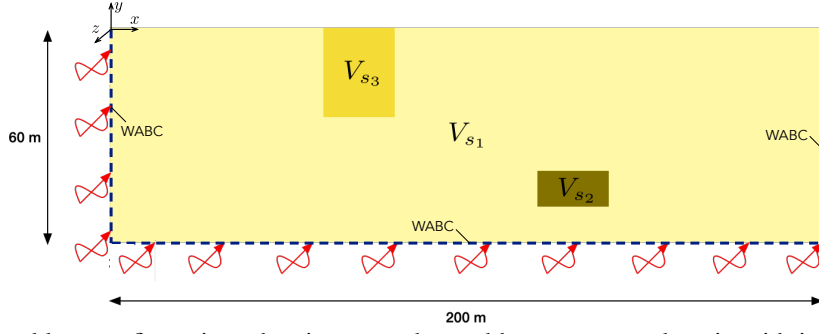


Figure 1: The problem configuration, showing an undamped homogeneous domain with its wave speed V_{s1} and inclusions with their wave speeds V_{s2} and V_{s3} .

been investigated in a truncated soil domain [2–7, 12, 15–19, 23–26, 28]. The inversion approach had also been used for estimating the most desirable, non-moving wave source profiles that can maximize wave energy in specific areas in solids [9–11, 20–22, 32]. To investigate the feasibility to identify arbitrary, incoherent incoming seismic waves in a truncated domain by using sparse seismic measurement by using the PDE-constrained optimization framework, [8, 13] studied a full-waveform source-inversion method to identify an incoming seismic wave in a 1D semi-infinite solid and a 2D bounded domain. Continuing the aforementioned papers [8, 13], the presented paper discusses a numerical method that can fully reconstruct the comprehensive profiles of complex, incoherent seismic incident wavefields propagating into a 2D truncated domain of anti-plane shear wave motions.

2 GOVERNING WAVE PHYSICS

Fig. 1 displays the problem configuration where the undamped domain is truncated by using a wave absorbing boundary condition (WABC), and the targeted, dynamic traction mimics a plane wave that impinges the domain with an incident angle of 45 degrees. The strong form of the governing differential equation for the shear wave propagation in the domain Ω , for time $t \in J = (0, T]$, is:

$$\nabla \cdot (G \nabla u) - \rho \frac{\partial^2 u}{\partial t^2} = 0, \quad \text{on } \Omega \times J, \quad (1)$$

where $u = u(x, y, t)$ denotes the displacement field in the anti-plane (z) direction of the wave motion of a solid particle; x and y denote the horizontal and vertical coordinates; $G(x, y)$ and $\rho(x, y)$ denote the shear modulus and the mass density of the solid.

The traction-free condition is presented on the top surface (Γ_t), while the absorbing boundary conditions are presented on the bottom (Γ_b), left (Γ_l) and right (Γ_r) boundaries:

$$\frac{\partial u}{\partial y}(x, 0, t) = 0, \quad 0 \leq x \leq L, \quad (2)$$

$$\frac{\partial u}{\partial y}(x, D, t) = \frac{f(x, D, t)}{G} - \frac{1}{V_s} \frac{\partial u}{\partial t}, \quad 0 \leq x \leq L, \quad (3)$$

$$\frac{\partial u}{\partial x}(0, y, t) = \frac{f(0, y, t)}{G} - \frac{1}{V_s} \frac{\partial u}{\partial t}, \quad D \leq y \leq 0, \quad (4)$$

$$\frac{\partial u}{\partial x}(L, y, t) = -\frac{1}{V_s} \frac{\partial u}{\partial t}, \quad D \leq y \leq 0, \quad (5)$$

where D is the y -coordinate of Γ_b ; L is the x -coordinate of Γ_r ; and $V_s(x, y)$ is the shear wave speed of the medium. We note that $f(x, y, t)$ denotes the dynamic traction of shear stress applied in the out-of-plane direction on the boundaries. Finally, the initial conditions are:

$$u(x, y, 0) = 0, \quad \frac{\partial u}{\partial t}(x, y, 0) = 0. \quad (6)$$

3 NUMERICAL MODELING

3.1 Finite element

The weak form of the forward problem is cast by multiplying the governing equation (1) by the test function v :

$$\begin{aligned} \int_{\Omega} \nabla v \cdot (G \nabla u) d\Omega + \int_{\Gamma_r} v \frac{G}{V_s} \frac{\partial u}{\partial t} d\Gamma + \int_{\Gamma_l} v \frac{G}{V_s} \frac{\partial u}{\partial t} d\Gamma + \int_{\Gamma_b} v \frac{G}{V_s} \frac{\partial u}{\partial t} d\Gamma \\ + \int_{\Omega} v \rho \frac{\partial^2 u}{\partial t^2} d\Omega = \int_{\Gamma_b} v f(x, y, t) d\Gamma + \int_{\Gamma_l} v f(x, y, t) d\Gamma, \end{aligned} \quad (7)$$

Then, by introducing the finite element approximations of the trial and test functions, the governing wave physics yields to the following discrete system:

$$\mathbf{K}\mathbf{u}(t) + \mathbf{C}\dot{\mathbf{u}}(t) + \mathbf{M}\ddot{\mathbf{u}}(t) = \mathbf{F}(t), \quad (8)$$

where $\mathbf{u}(t)$ denotes the solution vector of the state problem at time t , and $\dot{\mathbf{u}}$ and $\ddot{\mathbf{u}}$ denotes, respectively, the first and second-order derivative of \mathbf{u} with respect to time. The matrices and vector in the semi-discrete equation (8) can be written as:

$$\begin{aligned} \mathbf{K} &= \int_{\Omega} G \left(\frac{\partial \psi}{\partial x} \frac{\partial \psi^T}{\partial x} + \frac{\partial \psi}{\partial y} \frac{\partial \psi^T}{\partial y} \right) d\Omega, \\ \mathbf{C} &= \int_{\Gamma_r} \frac{G}{V_s} \psi \psi^T d\Gamma + \int_{\Gamma_l} \frac{G}{V_s} \psi \psi^T d\Gamma + \int_{\Gamma_b} \frac{G}{V_s} \psi \psi^T d\Gamma, \\ \mathbf{M} &= \int_{\Omega} \rho \psi \psi^T d\Omega, \\ \mathbf{F} &= \int_{\Gamma_b} \psi f d\Gamma + \int_{\Gamma_l} \psi f d\Gamma. \end{aligned} \quad (9)$$

3.2 Compact form

We solve (8) by using the implicit Newmark time integration. Although the time-integration steps are omitted, the procedure can be summarized in the following equation:

$$\mathbf{Q}\hat{\mathbf{u}} = \hat{\mathbf{F}}, \quad (10)$$

where the vector $\hat{\mathbf{u}}$ corresponds to the solution vector to all the time steps (i.e., the space-time discretization of $u(x, y, t)$); and $\hat{\mathbf{F}}$ is the global force vector. These vectors are built as:

$$\hat{\mathbf{u}} = \begin{bmatrix} \mathbf{u}_0 \\ \dot{\mathbf{u}}_0 \\ \ddot{\mathbf{u}}_0 \\ \vdots \\ \mathbf{u}_N \\ \dot{\mathbf{u}}_N \\ \ddot{\mathbf{u}}_N \end{bmatrix}, \quad \hat{\mathbf{F}} = \begin{bmatrix} 0 \\ 0 \\ \mathbf{F}_0 \\ \vdots \\ \mathbf{F}_N \\ 0 \\ 0 \end{bmatrix}, \quad (11)$$

where N is the number of time steps, and \mathbf{u}_j are the spatial degrees of freedom at the j -th time step. Finally, the matrix \mathbf{Q} is defined as:

$$\mathbf{Q} = \begin{bmatrix} \mathbf{I} & 0 & 0 & 0 & 0 & 0 & \dots & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & \mathbf{I} & 0 & 0 & 0 & 0 & \dots & 0 & 0 & 0 & 0 & 0 & 0 \\ \mathbf{K} & \mathbf{C} & \mathbf{M} & 0 & 0 & 0 & \dots & 0 & 0 & 0 & 0 & 0 & 0 \\ \mathbf{L}_1 & \mathbf{L}_2 & \mathbf{L}_3 & \mathbf{Keff} & 0 & 0 & \dots & 0 & 0 & 0 & 0 & 0 & 0 \\ a_1 \mathbf{I} & \mathbf{I} & 0 & -a_1 \mathbf{I} & \mathbf{I} & 0 & \dots & 0 & 0 & 0 & 0 & 0 & 0 \\ a_0 \mathbf{I} & a_2 \mathbf{I} & \mathbf{I} & -a_0 \mathbf{I} & 0 & \mathbf{I} & \dots & 0 & 0 & 0 & 0 & 0 & 0 \\ \vdots & \vdots & \vdots & \vdots & \vdots & \vdots & \ddots & \vdots & \vdots & \vdots & \vdots & \vdots & \vdots \\ 0 & 0 & 0 & 0 & 0 & 0 & \dots & \mathbf{L}_1 & \mathbf{L}_2 & \mathbf{L}_3 & \mathbf{Keff} & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & \dots & a_1 \mathbf{I} & \mathbf{I} & 0 & -a_1 \mathbf{I} & \mathbf{I} & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & \dots & a_0 \mathbf{I} & a_2 \mathbf{I} & \mathbf{I} & -a_0 \mathbf{I} & 0 & \mathbf{I} \end{bmatrix}, \quad (12)$$

where:

$$\begin{aligned} a_0 &= \frac{4}{(\Delta t)^2}, a_1 = \frac{2}{\Delta t}, a_2 = \frac{4}{\Delta t}, \\ \mathbf{Keff} &= a_0 \mathbf{M} + a_1 \mathbf{C} + \mathbf{K}, \quad \mathbf{L}_1 = -a_0 \mathbf{M} - a_1 \mathbf{C}, \\ \mathbf{L}_2 &= -a_2 \mathbf{M} - \mathbf{C}, \quad \mathbf{L}_3 = -\mathbf{M}. \end{aligned} \quad (13)$$

4 INVERSE MODELING

The presented work studies the discretize-then-optimize (DTO) approach. Under this inversion method, we identify P_{kj} —a component of $\hat{\mathbf{F}}$ corresponding to γ_k and t_j —as control parameters, where γ_k is the k -th discrete node on the boundaries Γ_1 and Γ_b . The discrete numbering of γ_k increases counter-clockwise from the top-left corner of the domain, and t_j is the j -th time step.

4.1 Discrete objective and Lagrangian functional

As a minimization problem, we attempt to determine the values of P_{kj} that minimize the discrete objective functional, which is defined as:

$$\hat{\mathcal{L}} = \frac{1}{2} (\hat{\mathbf{u}} - \hat{\mathbf{u}}_m)^T \bar{\mathbf{B}} (\hat{\mathbf{u}} - \hat{\mathbf{u}}_m), \quad (14)$$

where $\hat{\mathbf{u}}$ is obtained by a set of estimated P_{kj} ; $\hat{\mathbf{u}}_m$ (i.e., the space-time discretization of $u_m(x, y, t)$ induced by a set of P_{kj} corresponding to targeted force vector) is artificially generated by using a pseudo target of

traction. In (14), $\bar{\mathbf{B}}$ is defined as $\Delta t \mathbf{B}$, where \mathbf{B} is a square matrix, of which components are all 0 except for those of the diagonal, having values of all 1, if they correspond to the degrees of freedom of sensor locations. In this work, the measured data is obtained from sparsely-distributed sensors.

To tackle the minimization problem, we use the Lagrange multiplier vector $\hat{\lambda}$ to impose (10) onto (14), casting the following Lagrangian functional:

$$\hat{\mathcal{A}} = \frac{1}{2}(\hat{\mathbf{u}} - \hat{\mathbf{u}}_m)^T \bar{\mathbf{B}}(\hat{\mathbf{u}} - \hat{\mathbf{u}}_m) - \hat{\lambda}^T (\mathbf{Q}\hat{\mathbf{u}} - \hat{\mathbf{F}}), \quad (15)$$

where $\hat{\lambda}$ is built as:

$$\hat{\lambda} = [\lambda_0^T, \dot{\lambda}_0^T, \ddot{\lambda}_0^T, \dots, \lambda_N^T, \dot{\lambda}_N^T, \ddot{\lambda}_N^T]^T. \quad (16)$$

4.2 Optimality conditions

To identify unknown target control parameters, the first-order optimality conditions should be fulfilled. The first condition, $(\partial \hat{\mathcal{A}} / \partial \hat{\lambda}) = 0$, will be automatically satisfied when we solve the discrete forward problem (10), which is also referred to as the discrete state equation:

$$\frac{\partial \hat{\mathcal{A}}}{\partial \hat{\lambda}} = -\mathbf{Q}\hat{\mathbf{u}} + \hat{\mathbf{F}} = 0. \quad (17)$$

As the second condition, $(\partial \hat{\mathcal{A}} / \partial \hat{\mathbf{u}}) = 0$, will be automatically fulfilled when the following adjoint problem is solved:

$$\frac{\partial \hat{\mathcal{A}}}{\partial \hat{\mathbf{u}}} = -\mathbf{Q}^T \hat{\lambda} + \bar{\mathbf{B}}(\hat{\mathbf{u}} - \hat{\mathbf{u}}_m) = 0. \quad (18)$$

We note that in order to solve the adjoint equation (18), we have to march backward in time because it includes \mathbf{Q}^T .

As the third condition, $(\partial \hat{\mathcal{A}} / \partial \hat{\mathbf{F}}) = 0$, will be satisfied when we solve the resulting control problem:

$$\frac{\partial \hat{\mathcal{A}}}{\partial \hat{\mathbf{F}}} = \hat{\lambda} = 0, \quad (19)$$

which implies that the inversion solution of $\hat{\mathbf{F}}$ should lead to the vanishing $\hat{\lambda}$. (19) also implies that $\frac{\partial \hat{\mathcal{A}}}{\partial \xi} = \frac{\partial \hat{\mathcal{L}}}{\partial \xi}$ is the component of the vector $\hat{\lambda}$ corresponding to the global node numbering and the time step of ξ . In other words, if ξ is P_{kj} , (19) becomes:

$$\nabla_{(\xi=P_{kj})} \hat{\mathcal{A}} = \nabla_{(\xi=P_{kj})} \hat{\mathcal{L}} = \begin{cases} \ddot{\lambda}(\gamma_k, t_j) & \text{when } j = 1, \\ \lambda(\gamma_k, t_j) & \text{when } j \geq 2, \end{cases} \quad (20)$$

where $\ddot{\lambda}(\gamma_k, t_j)$ or $\lambda(\gamma_k, t_j)$ denotes the component of $\hat{\lambda}$ corresponding to the degree of freedom and the timestep of P_{kj} .

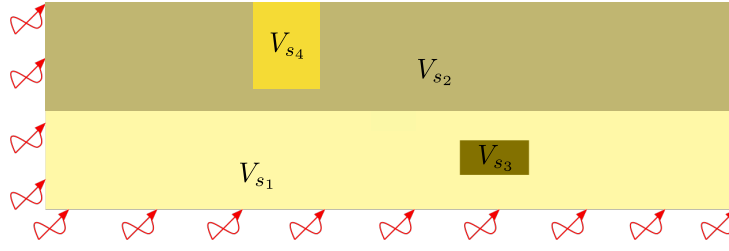


Figure 2: Material profile 2: a 2-layered solid with two inclusions.

4.3 Control parameters updates

In a gradient-based scheme, similar to [8], the control parameter vector ξ consisting of P_{kj} is updated iteratively as follows:

$$\xi_{(s+1)} = \xi_{(s)} + h_{(s)} \mathbf{d}_{(s)}, \quad (21)$$

where $h_{(s)}$ is a step length and $\mathbf{d}_{(s)}$ is a search-direction vector for $\xi_{(s)}$, and s is the inversion iteration index. Moreover, in every inversion iteration, Newton's method [8, 27] is utilized to establish the optimal $h_{(s)}$.

5 NUMERICAL EXPERIMENTS

In this section, three numerical experiments are considered by using a domain, which size is set to be $200 \text{ m} \times 60 \text{ m}$, and the mass density ρ is uniform as 3000 kg/m^3 . Moreover, two material profiles are considered in the presented numerical experiments. The material profile 1, as shown in Fig. 1, is of a homogeneous background solid domain, with a shear wave speed of $V_{s1} = 353.55 \text{ m/s}$, and it contains two inclusions, with $V_{s2} = 800 \text{ m/s}$, and $V_{s3} = 1000 \text{ m/s}$. The material profile 2, as shown in Fig. 2, is a 2-layered background solid with the same two inclusions. The shear wave speeds of material profile 2 are $V_{s1} = 353.55 \text{ m/s}$, $V_{s2} = 223.6 \text{ m/s}$, $V_{s3} = 800 \text{ m/s}$, and $V_{s4} = 1000 \text{ m/s}$, respectively.

The element size is set to be 0.5 m for computing the synthetic measured data $\hat{\mathbf{u}}_m$, while an element size of 1 m is used for evaluating state solution $\hat{\mathbf{u}}$ and the adjoint solution $\hat{\lambda}$ for an estimated force vector. However, the same Δt of 0.001 s is used for computing all $\hat{\mathbf{u}}_m$, $\hat{\mathbf{u}}$, and $\hat{\lambda}$.

For the material profile 1, two targeted force vectors $\hat{\mathbf{F}}^{1,3}$ (corresponding to two targeted, dynamic tractions applied on the left and bottom boundaries of the domain) are considered in this section. The time-dependent values of targeted $\hat{\mathbf{F}}^1$ corresponding to a node at a specific location $x = 100 \text{ m}$ and $y = -60 \text{ m}$ and its frequency content—a Ricker wavelet with a central frequency of 5 Hz —are shown in Fig. 3(a-b). The time-dependent signal corresponding to $\hat{\mathbf{F}}^3$ at the same location, and its frequency content is shown in Fig. 3(c-d). This six-second length signal was extracted from the ground motion database of the Pacific Earthquake Engineering Research Center (PEER), with this portion being recorded during the 1994 Northridge earthquake.

On the other hand, $\hat{\mathbf{F}}^2$ is a force vector due to dynamic traction on the left and bottom boundaries of a two-layered background domain (i.e., the material profile 2), and this signal mimics incident plane wavefield of an incident angle 45° in the bottom layer. The signal corresponding to $\hat{\mathbf{F}}^2$ at $x = 100 \text{ m}$ and $y = -60 \text{ m}$ is a Ricker wavelet with a central frequency of 5 Hz .

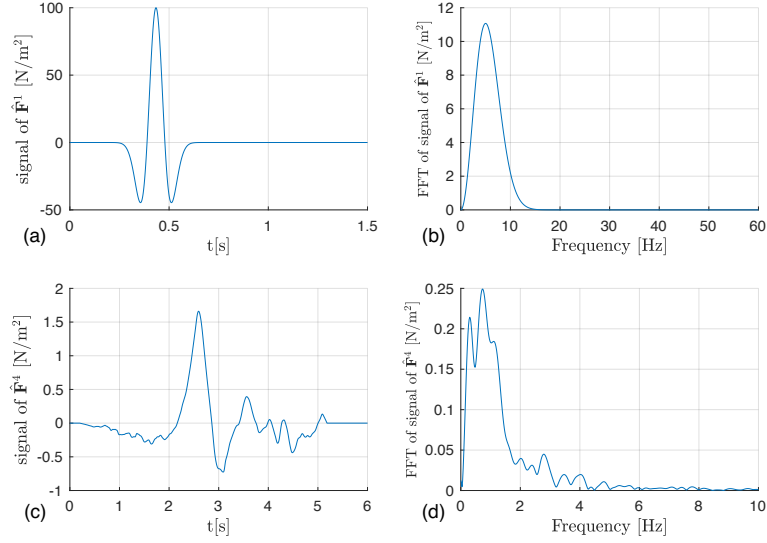


Figure 3: (a,c) The time signals of $\hat{\mathbf{F}}^{1,3}$ corresponding to a node at $x = 100$ m and $y = -60$ m, respectively; (b,d) the amplitude of Fourier Transform of each signal.

Cases 1 and 2, which consider $\hat{\mathbf{F}}^{1,2}$, use 391,791 control parameters, while Case 3, which consider $\hat{\mathbf{F}}^3$, uses 1,566,261 control parameters. Prior to the first inversion iteration, the control parameters are set as zero.

For appraising the effectiveness to reconstruct $\hat{\mathbf{F}}$ in the numerical results, we compare the components of $\hat{\mathbf{F}}_{\text{target}}$ and $\hat{\mathbf{F}}_{\text{estimate}}$ corresponding to the nodes on the boundaries where tractions are applied. Therefore, the following error norm is used:

$$\mathcal{E} = \frac{|\hat{\mathbf{F}}_{\text{target}} - \hat{\mathbf{F}}_{\text{estimate}}|^2}{|\hat{\mathbf{F}}_{\text{target}}|^2} \times 100[\%]. \quad (22)$$

5.1 Example 1: Inversion performance with respect to the background domain complexity.

In this example, the performance of inverting a targeted set of P_{kj} of $\hat{\mathbf{F}}^1$ by considering a single-layered background domain with inclusions (i.e., Case 1) is compared with the performance of inverting a targeted set of P_{kj} of $\hat{\mathbf{F}}^2$ using the two-layered, heterogeneous background domain counterpart (i.e., Case 2). To this end, the traction signals of both $\hat{\mathbf{F}}^1$ and $\hat{\mathbf{F}}^2$ have the same central frequency of 5 Hz, and the same sensor configuration is used in both Cases—50 sensors on Γ_t and a vertical array of 15 sensors on Γ_r . The great agreement between the targeted P_{kj} and its estimated equivalent is indicated in Fig. 4 for both Cases. Fig. 5 shows the value of \mathcal{E} over iterations in Case 1 and 2 for material profiles 1 and 2, respectively. Despite using a more complex background domain in Case 2, the difference between the terminal values of \mathcal{E} in Cases 1 and 2 is only 0.11%.

Thus, we suggest that the accuracy of our presented inversion algorithm is not compromised by the material complexity of a background domain.

5.2 Example 2: Inversion performance with respect to the signal complexity.

This example investigates the performance of identifying a targeted set of P_{kj} of $\hat{\mathbf{F}}^3$, which is created

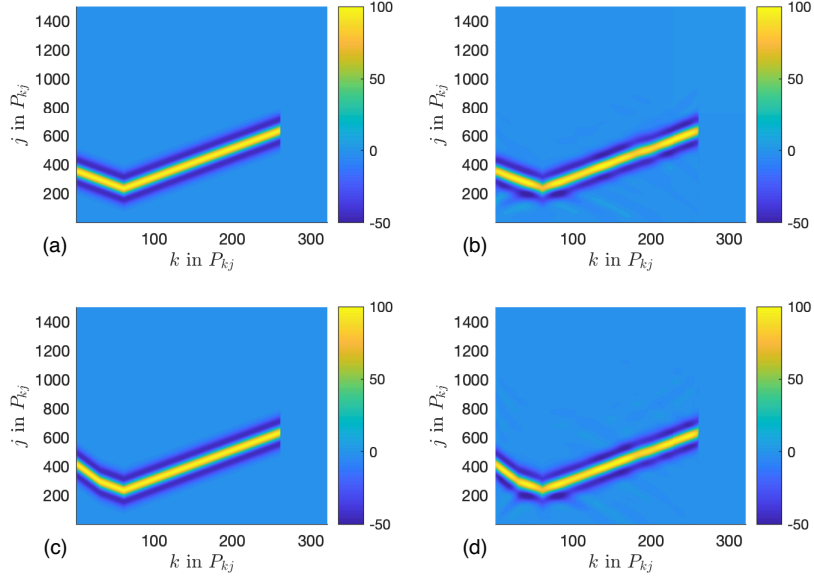


Figure 4: Example 1: (a) Target and (b) Reconstructed P_{kj} for Case 1; and (c) Target and (d) Reconstructed P_{kj} for Case 2 at the 1000th iteration.

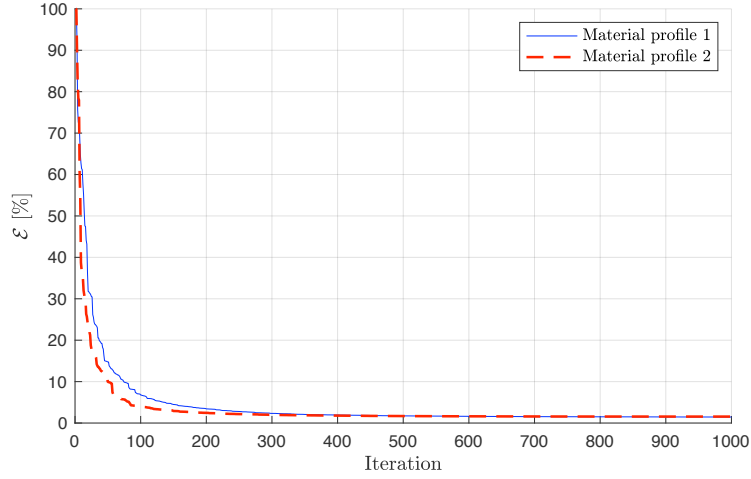


Figure 5: Example 1 - The final value of \mathcal{E} , in Case 1, is 1.45%, and that in Case 2 is 1.56%.

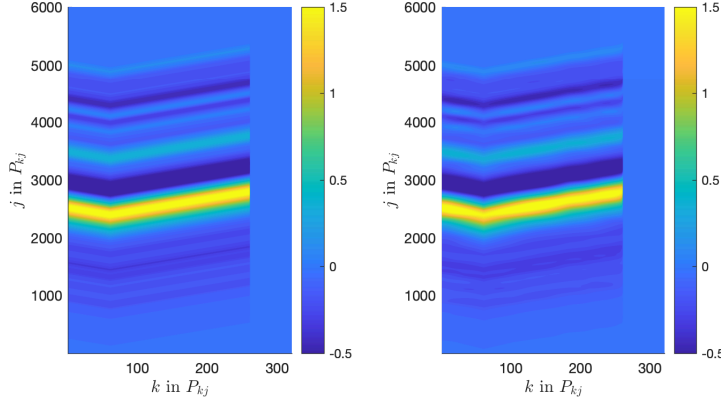


Figure 6: Example 2: (a) A target set of P_{kj} and (b) its reconstructed counterpart corresponding to $\hat{\mathbf{F}}^2$ for Case 3 at the 1000th iteration.

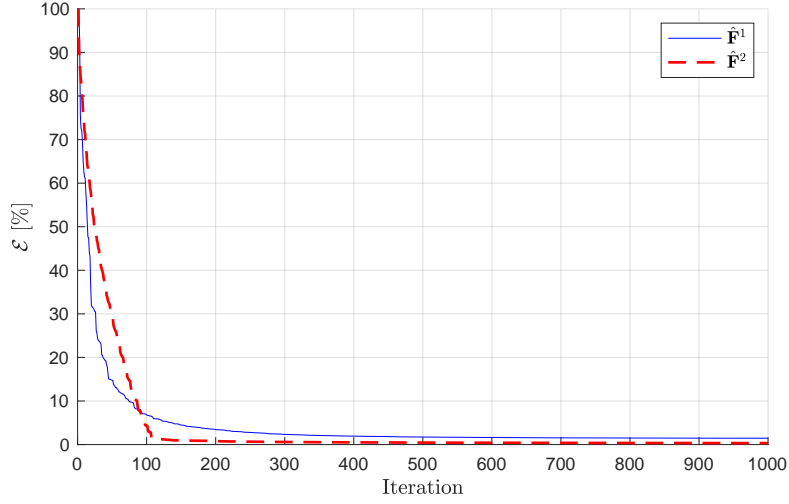


Figure 7: Example 2 - \mathcal{E} over iterations in Case 1 and Case 2.

by a realistic seismic signal (Case 3). In Case 3, we use the material profile 1 with 50 sensors on Γ_t and 15 sensors on Γ_r . Fig. 6 compares the targeted and reconstructed P_{kj} in Case 3, and it shows the excellent concordance between them. The inversion performance in Case 3 is also compared with that in Case 1. Cases 1 and 3 use the same material and sensor configurations except for different force vectors $\hat{\mathbf{F}}^1$ and $\hat{\mathbf{F}}^3$, respectively. Fig. 7 shows that the final value of \mathcal{E} , 0.33%, in Case 3 is smaller than that in Case 1 (1.45%). We note that, even though the signal to generate $\hat{\mathbf{F}}^3$ is more complex than the other corresponding to $\hat{\mathbf{F}}^1$, our inversion solver identifies $\hat{\mathbf{F}}^3$ more accurately than $\hat{\mathbf{F}}^1$ because the signal in $\hat{\mathbf{F}}^3$ has a lower dominant frequency than $\hat{\mathbf{F}}^1$. Thus, we suggest that the presented inverse modeling experiences less solution multiplicity when it reconstructs lower frequency traction (e.g., a realistic seismic signal), as discussed in [8].

6 CONCLUSIONS

In this study, we used a PDE-constrained optimization method to reconstruct incoherent dynamic traction in a 2D domain truncated by wave-absorbing boundary conditions (WABC). Specifically, we used the dynamic traction at the WABC to mimic seismic incident wavefield, the optimization method is utilized to tackle the inverse problem, and the DTO approach is employed to solve the adjoint problem. It was shown that the target traction can be reconstructed by using the presented inverse method. We also showed that the traction signals are well reconstructed despite the complexity of the background domain. Additionally, we showed that the presented minimizer can reconstruct a realistic seismic signal, and it suffers less when identifying signals with a lower frequency content.

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