Durability Assessment of GFRP Rebars Exposed to High pH-Seawater

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Abstract. There is a continued process to implement innovative materials to enhance the sustainability and durability of the built infrastructure. Technologies developed over the last two decades have facilitated the use of glass fiber reinforced polymer (GFRP) composites as internal reinforcement bars (rebars) for concrete structures, which have proven to be an alternative to traditional steel reinforcement due to significant advantages, such as magnetic transparency and, most importantly, corrosion resistance, equating to durability and structural life extension. This study evaluates the durability of three different available and most commonly used GFRP rebar types, based on exposure to aggressive environments, such as those experienced in coastal areas. For that, the specimens were expose to high pH seawater solution (that simulates the alkalinity of the concrete exposed to seawater), at 60 °C for different periods of time: 45, 90, and 180 days. The durability of these GFRP rebars was assessed by testing four different physio-mechanical properties, including: tensile strength, elastic modulus, and transverse and horizontal shear strength. Preliminary results show that the resilience of the GFRP rebars after being exposed to high pH seawater at high temperature, varies considerably among the three different types. The tensile strength was the most affected physio-mechanical property.

Keywords: Composites, Durability, GFRP Rebars, Resilience.

1 Introduction

The use of GFRP bars (Glass Fiber Reinforced Polymer) is gaining importance as internal reinforcement for reinforced concrete (RC) structures, primarily due to the corrosion-resistant properties. One of the most important applications for this alternative reinforcement is its use in coastal RC structures where corrosion of traditional steel rebars is critical (Nolan, Rossini and Nanni, 2018) and poses significant issues for the longevity and cost of the infrastructure. This is especially important in marine environments due to: i) salt water in direct contact with concrete structures, through foundations or by air and ii) the need to increase the infrastructure's future resilience and sustainability to sustain the numerous effects related to climate change, such as the sea level rise. These critical needs may no longer be of concern with the use of

GFRP bars; which are composite reinforcing bars made of glass fibers embedded in a resin matrix and are non-corrosive. Because the driving force for such alternative reinforcement bars is the lifespan extension of RC structures, the durability of GFRP rebars must be carefully evaluated, as these aspects are fundamental for the expansion and extended application of this technology.

Concrete is considered a highly alkaline material, with typical pH values for freshly placed concrete above 12.5 to 13.9 for concrete made with high alkaline cement (Grubb, Jennifer A., *et al.*, 2007). When using steel reinforcement, the high alkalinity of the concrete provides corrosion protection to the steel by providing a passive layer of iron oxide (Fe₂O₃). Throughout the service life of the reinforced concrete structures, different phenomena such as carbonation, ingress of corrosive agents through voids and cracks, etc. lead to the breakdown of this passive layer, favouring the corrosion of steel. With GFRP rebars, however, the high alkalinity of concrete does not have a protective effect as for the steel, but it appears to affects the durability of the rebars in a negative way instead.

In the literature, different studies exist that have proven the deterioration of the tensile properties of GFRP rebars embedded in concrete or subjected to high pH solutions which simulate the alkalinity of the concrete, through accelerated aging protocols (Dejke and Tepfers, 2001; Chen, Davalos and Ray, 2006; Robert, Cousin and Benmokrane, 2009; Robert and Benmokrane, 2013; Wang, X.-L. Zhao, *et al.*, 2017). In addition, Wang *et al.* (2017) evaluated the effect of the simulated seawater and sea sand concrete on the horizontal shear properties of GFRP rebars, while Ruiz Emparanza *et al.* (2018) Yan *et al.* (2017) and Dong *et al.* (2016) assessed the resilience of bond properties of GFRP rebars embedded in concrete and exposed to seawater. Each of these studies are focused on the resilience of a single mechanical property, and to the best knowledge of the author, no research exists that integrates a combined durability assessment of different mechanical characteristics, which would help to have a better understanding of the degradation mechanism affecting GFR bars.

Therefore, this study is comprised by the evaluation of the durability of three different commercially available GFRP rebars exposed to a combination of high pH and seawater, simulating coastal reinforced concrete structures. The rebars were aged for up to 180 days at 60°C to accelerate the degradation process. High temperature is used since it is expected that the rate of reaction will double every 10°C (Pauling 1988). A temperature of 60°C was chosen based on the practices found in the literature (Dejke and Tepfers, 2001; Chen, Davalos and Ray, 2006; Robert, Cousin and Benmokrane, 2009; Robert and Benmokrane, 2013; Wang, X.-L. Zhao, *et al.*, 2017). Four different mechanical properties were tested: tensile strength, modulus of elasticity, transverse shear strength and horizontal shear strength.

2 Experimental Program

Three types of GFRP bars, denoted as A, B and C, were tested for comparison purposes due to diversity of the products in the GFRP rebar market (Ruiz Emparanza, Kampmann and De Caso Y Basalo, 2017). All tested bars in this study had a nominal diameter of 10-mm and were made of continuous EC-R glass fiber and impregnated with a vinyl ester resin. However, the manufacturing process and surface enhancements varied among them: Type-A bars were sand-coated, Type-B were helically wrapped, while Type-C were ribbed (see Figure 1).



Figure 1. GFRP rebars Type A, B and C (from left to right).

The focus of this study was to assess the mechanical performance of three GFRP bar types before and after being aged in simulated seawater concrete pore solution. Four different mechanical properties such as tensile strength, modulus of elasticity and transverse and horizontal shear strength were evaluated. For every property and exposure condition, a minimum of three specimens were tested. Table 1 summarizes the test matrix with the corresponding standardized test method used in this study.

Rebar	Temp.	Duration		Test Method	
type	°C	Days	Mechanical Property		
A B C	60	45 90 180	Tensile Strength	ASTM D7205	
			Modulus of Elasticity		
			Transverse Shear Strength	ASTM D7617	
			Horizontal Shear Strength	ASTM D4475	

Table 1.	Test Matrix	for	durability	assessment.
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In addition to the tests run on specimens that were aged at 60 °C for 45, 90 and 180 days as seen in Table 1, three repetitions per mechanical property were tested on non-exposed specimens. These results were used as benchmark to assess the resilience of the mechanical properties after the accelerated aging protocol.

2.1 Simulated Environmental Exposure Condition

The accelerated aging of the GFRP bars specimens was achieved by completely immersing the bars into a to high pH seawater solution at 60 °C and different exposure times (45, 90 and 180 days). The seawater used was directly taken from Key Biscayne (Florida, USA). The solution was made by dissolving calcium hydroxide, sodium hydroxide and potassium hydroxide in seawater (2 g of Ca(OH)₂ + 2.4 g of NaOH + 19.6 g of KOH per liter of seawater). The mass concentrations of the solution were chosen according to Chen et. al 2006 (Chen, Davalos and Ray, 2006). The high alkalinity solution simulates the pore solution of normal concrete with a pH value of about 13.6.

2.3 Specimen Preparation

After each exposure condition, the GFRP rebars were extracted from the solution and prepared for testing according to the corresponding ASTM standards. The preparation of the specimens used to test the transverse shear strength consisted of cutting the rebars to 230 mm specimens while the samples for horizontal shear were cut to a length of 6 times the diameter (60 mm). The preparation of the tensile samples to assess the ultimate tensile strength and modulus of elasticity, was more extensive: after the rebars were cut to length of 1000 mm, protective anchors were installed at both ends of the samples to protect the rebar when being gripped by the loading machine. The anchorage system was accomplished by a 300 mm long steel pipe filled with expansive grout which will reduce lateral stresses during griping, since GFRP rebars are weak in the transverse direction compared to the longitudinal one.

2.3 Testing Procedure

Each of the tests were conducted in accordance with the corresponding ASTM: the test setups are shown in Figure 2.



Figure 2. Test setup: horizontal shear test (top left), transverse shear test (bottom left), and tensile test (right).

In this study, all tests were performed by applying displacement-controlled loading using a Baldwin machine with a capacity of 890-kN for the tensile test, and a 133-kN Instron test frame

for transverse and horizontal shear tests. The test rate for the tensile test was 2 mm/min, while for transverse and horizontal shear test specimens were tested at a rate of 1.3 mm/min. As per the corresponding ASTMs (see Table 1), for transverse and horizontal shear tests, the crosshead displacement and the load were recorded. For the tensile tests, in addition to the load, each of the specimens were instrumented with a 100 mm long extensometer to record the strain, which was then used to determine the modulus of elasticity.

3 Results and Discussion

For the three different types of rebars within the scope of this research project, four mechanical properties of unaged and aged specimens were evaluated.

3.2 Unaged Samples

The mechanical properties for 'as-received' GFRP bars used in this study are summarized in Table 2. Mechanical characterization of pristine GFRP bars provided unconditioned values as a reference for residual strength after exposure.

Rebar	Tensile Strength		E-Modulus		Transverse Shear Strength		Horizontal Shear Strength	
type	Avg.	CoV	Avg.	CoV	Avg.	CoV	Avg.	CoV.
	MPa	%	MPa	%	MPa	%	MPa	%
А	953.8	5.5	51570	5.1	212.4	4.8	42.9	7.7
В	834.0	5.5	55340	2.0	194.5	3.3	45.3	5.4
С	982.5	4.3	54570	6.0	211.2	2.6	50.3	5.6

Table 2. Nominal mechanical properties of 10mm diameter reference GFRP bars.

3.2 Aged samples

After being exposed to high pH seawater solution for 45, 90 and 180 days, the aged samples were tested, and the results obtained for each of the four mechanical properties were compared to the benchmark values obtained from the unaged specimens, to obtain the retention over time, as shown in Figure 3 ,4 and 5.

For all the rebar types, the tensile strength was the most affected property with a reduction of up to 41% for Type-A, while the tensile capacity for Type B and C decreased 29% and 20% respectively. Compared to the literature, Chen *et al.* (2006) saw higher degradation (about 70% of degradation after 180 days) when exposing GFRP rebars to the same solution (except they used tab water instead of seawater to mixed with the same proportions of hydroxides).



Figure 3. Retention of the tested mechanical properties for Type-A rebars.

This could be due to the lesser development of the GFRP products back in 2006. More recent research conducted by Robert *et al.* (2013) showed a lower reduction (about 10%), but the simulated seawater pore solution had a lower pH (12.15 compared to 13.5 of the current study) and the aging temperature was of 50 °C instead of 60°C.



Figure 4. Retention of the tested mechanical properties for Type-B rebars.

The second most affected property was the modulus of elasticity, even if the reduction was relatively low: between 5% and 13%. These values are aligned with the data found in the literature (Robert, Cousin and Benmokrane, 2009; Robert and Benmokrane, 2013), where it was seen that that degradation of the modulus was insignificant compared to the reduction of the maximum tensile capacity. It is believed by the authors that a degradation of the fiber and resin interface takes place with the time, which affects the stress transfer between fibers. This reduction is more noticeable at a critical strain (related to the maximum tensile strength) where the decrease in the stress transfer capacity may lead to the inability to activate all the fibers of the cross section of the rebar which will result in higher strains in those fibers that are being engaged. This will then cause failure of individual fibers at an earlier stage, leading to a premature failure of the rebar. However, since the elastic modulus is not related to individual

or incremental fiber failures but rather to the sum of the elastic strain all along the coupon or rebar gage length, is not that heavily affected.



Figure 5. Retention of the tested mechanical properties for Type-C rebars.

Finally, for all the rebar types, the deterioration in terms of transverse and horizontal shear capacity was insignificant. Similar behavior has been reported by Benmokrane *et al.* (2015), in which GFRP bars (vinyl ester resin matrix with E-glass fibers) where exposed to alkaline solution (pH 12.6 - 13.0) and resulted in horizontal and transverse shear strength reduction of 5% and 10%, respectively, after 5,000 h at 60°C. This was also seen by Dejke *et al.* (2001), even though Wang *et al.* (2017) saw a decrease in the horizontal shear capacity after the rebars being aged in a high pH solution. This decrease can be related to the use of different types of rebars, specially to the type of fiber the rebars tested by Wang *et al.* (2017): E-Glass was used instead of E-CR Glass, being this last one the upgraded version of E-Glass, with corrosion resistance features.

4 Conclusions

In this study, the durability of three different GFRP rebar types was assessed after being exposed to a high pH seawater solution at 60 °C and different exposure times (45, 90 and 180 days). Four different mechanical properties were tested: tensile strength, modulus of elasticity, transverse shear strength and horizontal shear strength. It was seen that the resilience after the accelerated exposure depended on the type of rebar. However, for all of them, the tensile strength was the most affected mechanical property, with a reduction of 20 and 41%, followed by the elastic modulus, which decreased between 5% and 13%. Finally, the horizontal and transverse shear strength capacities were almost unaltered after the aging process. These preliminary results were aligned with the findings reported by other researchers in the literature.

Regarding the significant difference in the reduction between the tensile strength compared to the modulus of elasticity, authors believe that it could be related to the degradation of the fiber and resin interface, which affects the stress transfer between fibers. This reduction is more noticeable at a critical strain (related to the maximum tensile strength) where the decrease in the stress transfer capacity may lead to the inability to activate all the fibers of the cross section of the rebar which will result in higher strains and, therefore, premature failure in those fibers that are being engaged. But, since the elastic modulus is not related to individual or incremental fiber failures but rather to the sum of the elastic strain all along the coupon or rebar gage length, is not that heavily affected.

However, the authors of the article acknowledge that more data is needed to confirm these preliminary conclusions. Future research activities include the extension of the aging time and addition of extra exposure temperatures. This data will be then used to model the durability and predict the service life. Finally, it needs to be noted that the durability prediction values should be compared to results obtained from existing structures, to calibrate the durability model.

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