

Behaviour of Surface Chloride Concentration in Concretes Subjected to Field Exposure in Marine Atmosphere Zone

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Abstract. Surface chloride concentration (C_s) is one of the parameters that feed models used to simulate chloride ingress into concrete. Therefore, understanding its behaviour over time is important for a more accurate forecasting. This work is part of a larger researcher project that aims to analyse the transport of chlorides into concrete in marine atmosphere zone based on long-term field exposures. The present paper focuses on the behaviour of C_s along 12.5 years. Prismatic concrete specimens with three different mixtures were exposed at places located at four different distances from the sea. Climatic variables and chloride deposition on the wet candle were parameters used to characterise the environment. Periodically, samples from concrete surface were extracted from the specimens and chemically analysed. Results show that C_s increases along the years and suggests a tendency of stabilisation over time, although this level could not be reached in the present exposure period. The relationship between C_s and chloride deposition rate on the wet candle was analysed and it was observed that the function $C_s = C_0 + k_{cs} \cdot (Dac)^n$ is the one that best fits to experimental data.

Keywords: Concrete, Corrosion, Marine Atmosphere Zone, Surface Chloride Concentration.

1 Introduction

Surface chloride concentration (C_s) is one of the main parameters to feed models used to simulate chloride penetration into concrete structures. It has been observed that C_s tends to increase over the years (Costa and Appleton, 1999; Yang *et al.*, 2017). However, this increase tendency weakens along time reaching a stabilisation, which can be observed after about ten years of exposure time in some cases (Andrade *et al.*, 2000). This behaviour can be represented by some mathematical models, which are presented in Table 1, that shows functions proposed in literature to represent the behaviour of C_s in concrete structures exposed in marine atmosphere zone.

It can be observed that there are not many studies aimed on the behaviour of C_s and that most of functions are power or exponential functions, with a predominance of first one.

Analysing the results presented by the studies referenced in Table 1 it can be observed that, in general, C_s sharply increases in the first years and, in the following years of exposure, this trend gives way to a more understated increase. Depending on the function adopted to represent

data behaviour, its final part can assume a more asymptotic shape, which denotes a stabilisation trend along the years. Moreover, in some cases C_s presents some fluctuation, which can be a result of the environmental interaction (Andrade *et al.*, 2000; Meira, 2004).

Table 1. Literature models to represent surface chloride concentration behaviour in marine atmosphere zone.

Source of data	Exposure time (years)	Function	Authors	Year
Japan	23 - 58	$C_s = a \cdot t^{0.5}$	Uji <i>et al.</i>	1990
Portugal	0.5 - 5.5	$C_s = a \cdot t^b$	Costa and Appleton	1999
United States of America	2 - 16	$C_s = a \cdot (1 - e^{b \cdot t})$	Kassir <i>et al.</i>	2002
South Korea	0.7 - 48.7	$C_s = a \cdot \ln(b \cdot t + 1) + c$	Pack <i>et al.</i>	2010
Data from literature	0 - 3	$C_s = a + b \cdot t^{0.5}$	Zhou <i>et al.</i>	2016
Data from literature	0 - 5	$C_s = a \cdot (1 - e^{b \cdot t})$	Yang <i>et al.</i>	2017

C_s = surface chloride concentration t = exposure time

Regarding the shape of C_s curve, the continuous cement paste hydration along time is one of the aspects that may influence its behaviour. As hydration of cement paste advances, the concrete surface becomes less porous and less chloride ions can be captured in this region (Maheswaram and Sanjayan, 2004; Pack *et al.*, 2010). As a consequence, the rate of C_s increase weakens. Besides that, the concrete ability in capturing chlorides decreases with the chloride concentration increase, which also contributes in the same way.

Another aspect to be considered in this analysis is the aggressiveness of the environment to which concrete structure is subjected. At places with a higher availability of chlorides there is a stronger increase of C_s in the first years with a subsequent attenuation with time (Sandberg *et al.*, 1998; Costa and Appleton, 1999). However, along the years, C_s may present some fluctuation, which can be related to the ions movement towards bulk concrete or to effects like surface chloride removal due to rainfall.

Although there are some proposals to represent C_s behaviour along time in marine atmosphere zone, they are still scarce and there is no consensus related to the best function to represent C_s behaviour. This work contributes to this discussion and analyses the behaviour of C_s in concretes exposed along 12.5 years at a marine atmosphere zone located in northeast of Brazil. This is part of a project that studies the long-term chloride transport into concrete under natural exposure in marine atmosphere zone.

2 Experimental Work

Experimental work was based on environmental characterisation and chloride concentration measurements in concrete surface.

2.1 Environmental Characterization

The environmental characterization was done on temperature, relative humidity, rainfall, wind characteristics and sea-salt data. Climatic data were collected by a Brazilian Government weather station located in the region where the research took place. Sea-salt data was collected

at places 10, 100, 200 and 500 m far from the sea (Figure 1 and Table 2) using the wet candle method, according to ASTM standard G140 (ASTM, 2014).



Figure 1. Region where wet candle devices and concrete specimens were exposed.

Table 2. Geographical coordinates of exposure sites in northeast of Brazil.

Distance from the sea (m)	Latitude	Longitude
10	7°1'42.9''	34°49'50.1''
100	7°1'42.2''	34°49'52.8''
200	7°1'41.0''	34°49'55.8''
500	7°1'47.2''	34°50'13.1''

2.2. Surface Chloride Concentration in Concrete

Prismatic concrete specimens (0.15 x 0.15 x 1.40 m) were cast using a filler-modified Portland Brazilian cement, which chemical and physical properties are presented in Table 3. Concrete mixtures, with w/b ratios between 0.65 to 0.50, and physical properties are presented in Table 4. They are identified as C65, C57 and C50. Considering that these mixtures were performed a long time ago, they considered w/b that are not usual nowadays.

Table 3. Chemical and physical properties of used cement.

Composition (%)	SO ₃	SiO ₂	Al ₂ O ₃	Fe ₂ O ₃	CaO	MgO	Na ₂ O	K ₂ O	Insoluble residue (IR)	Loss on ignition (LI)
	3.21	18.11	4.31	2.27	59.87	3.61	0.21	1.51	1.45	5.50
Property	Specific surface (cm ² /g)			Specific density (g/cm ³)						
	3650			3.06						

Table 4. Concrete mixtures and properties.

Concrete	C50	C57	C65
<i>Mixture</i>			
Cement (kg/m ³)	406	356	320
Sand (kg/m ³)	769	812	840
Coarse aggregate (kg/m ³)	947	947	947
Plasticiser (kg/m ³)	1.22	1.06	-
w/b	0.5	0.57	0.65
<i>Property</i>			
Slump (mm)	80	80	80
Compressive strength (MPa – 28 days)	31	27	20

The specimens were cured in a wet chamber for 7 days and afterwards they were painted with a waterproof film at those surfaces thorough which chloride penetration should be avoided. Then, the specimens were placed at the same monitoring stations used for wet candle devices. After 6, 10, 14, 18, 46, 78 and 150 months of exposure, samples were extracted from the specimens to obtain chloride profiles in concrete. Although chloride profiles were obtained at each sampling period, here only surface chloride contents are analysed. These samples were extracted in the first millimetre of the exposure surfaces of specimens. The total chloride content was determined by potentiometric titration, following the procedures of the International Union of Laboratories and Experts in Construction Materials, Systems and Structures (RILEM, 2002).

3 Results and Analysis

Climatic results show that temperature ranged between 16 and 33.6 °C along this period, with an average value of 27 °C. The relative humidity presented a fluctuation between 55 and 99 %, with an average value of 76.6 %. Higher values were reached during the winter (rainy season) that takes place mainly between May and August. Wind speed data ranged between 1.5 and 7.6 m/s, with average value in this period around 3 m/s. Predominant wind directions were south (S), southeast (SE) and East (E), with a preponderance of SE winds. This is a typical behaviour for the studied region (Meira *et al.*, 2006).

Average chloride deposition data are presented in Figure 2. This figure shows a strong salinity decrease in the first meters far from the sea, which may affect reinforced concrete structures in different levels. This drop in salinity is a consequence of the removal of marine aerosol salt particles due to gravimetric effect joined with other removal mechanisms (obstacles, rain, etc.).

Results of the surface chloride concentration are presented in Figure 3, considering the three different concretes and the four exposure sites and their distances from shoreline. Regarding the general aspects of the data, they show some fluctuation in the first exposure months, followed by a period of significant increase and afterwards a tendency of increase at lower rates, which suggests a tendency of reaching a maximum along time, but that was not possible to be observed in these 12.5 years of exposure. The initial fluctuation of chloride concentration in concrete surface can be attributed to the environmental interaction, where the rainfall can play an important role, due to the wash-out effect [Meira 2004, Chen *et al.*, 2013].

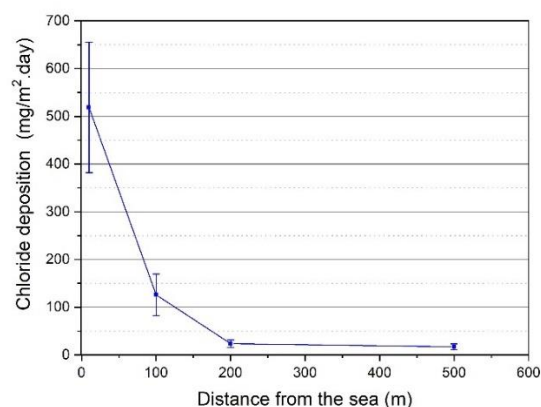


Figure 2. Average chloride deposition data during the research period.

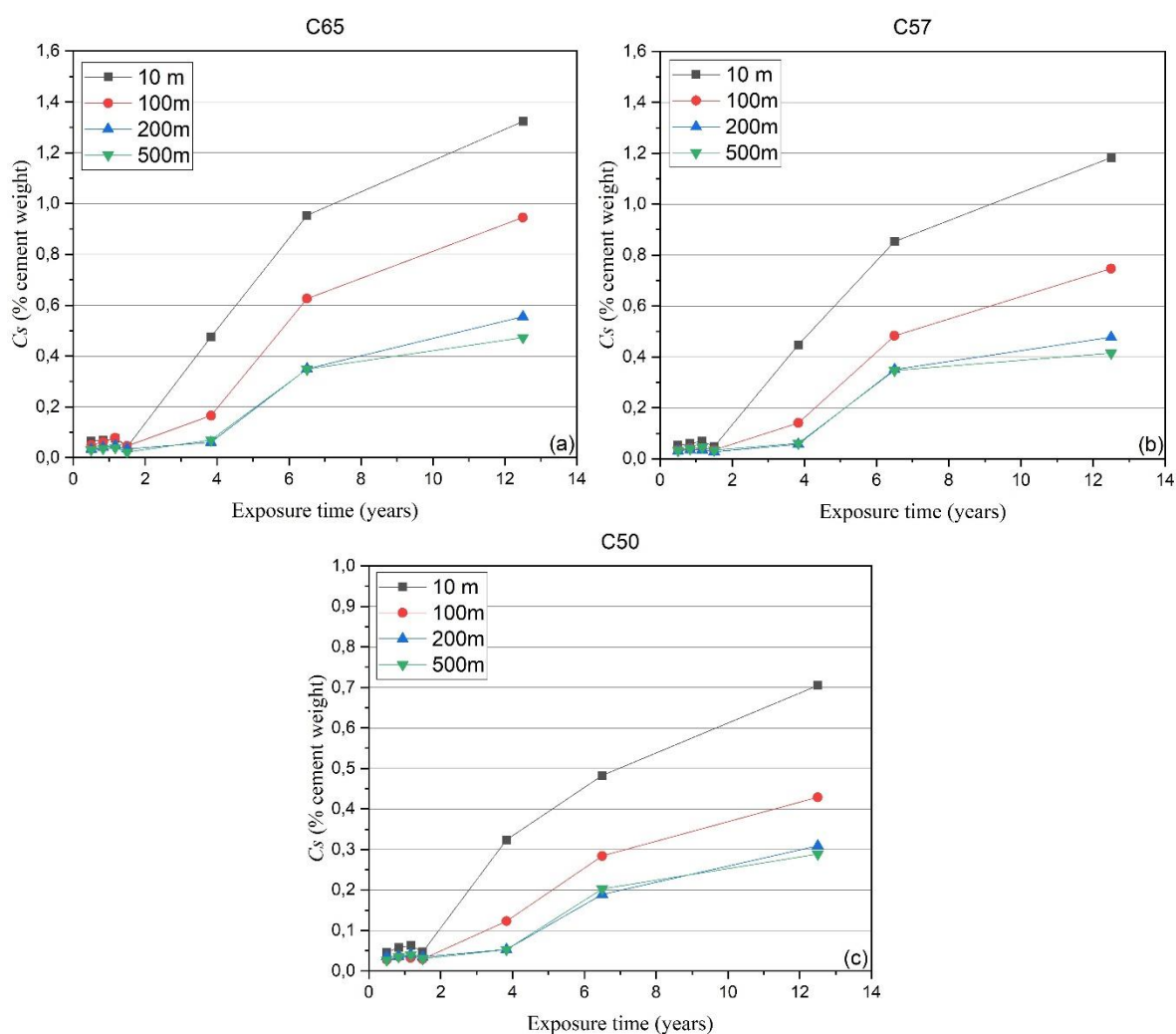


Figure 3. Behaviour of C_s along time for concretes C65 (a), C57 (b) and C50 (c).

Regarding the influence of concrete characteristics, it is clear the higher accumulation of chlorides in concrete surface as concrete porosity increases (higher w/b). Moreover, taking into account the distance from the sea, it is noticeable the chloride concentrations decay as going far from the sea, which is a consequence of the aggressiveness decrease at sites far from the shoreline due to the lower availability of chlorides in atmosphere.

Considering that the time is not the main variable that influences C_s increase, but the availability of chlorides in atmosphere, it seems quite reasonable to analyse the relationship between C_s and the accumulated deposition of chlorides on the wet candle (Dac), which is obtained summing month-to-month the chloride deposition on the wet candle (Figure 4).

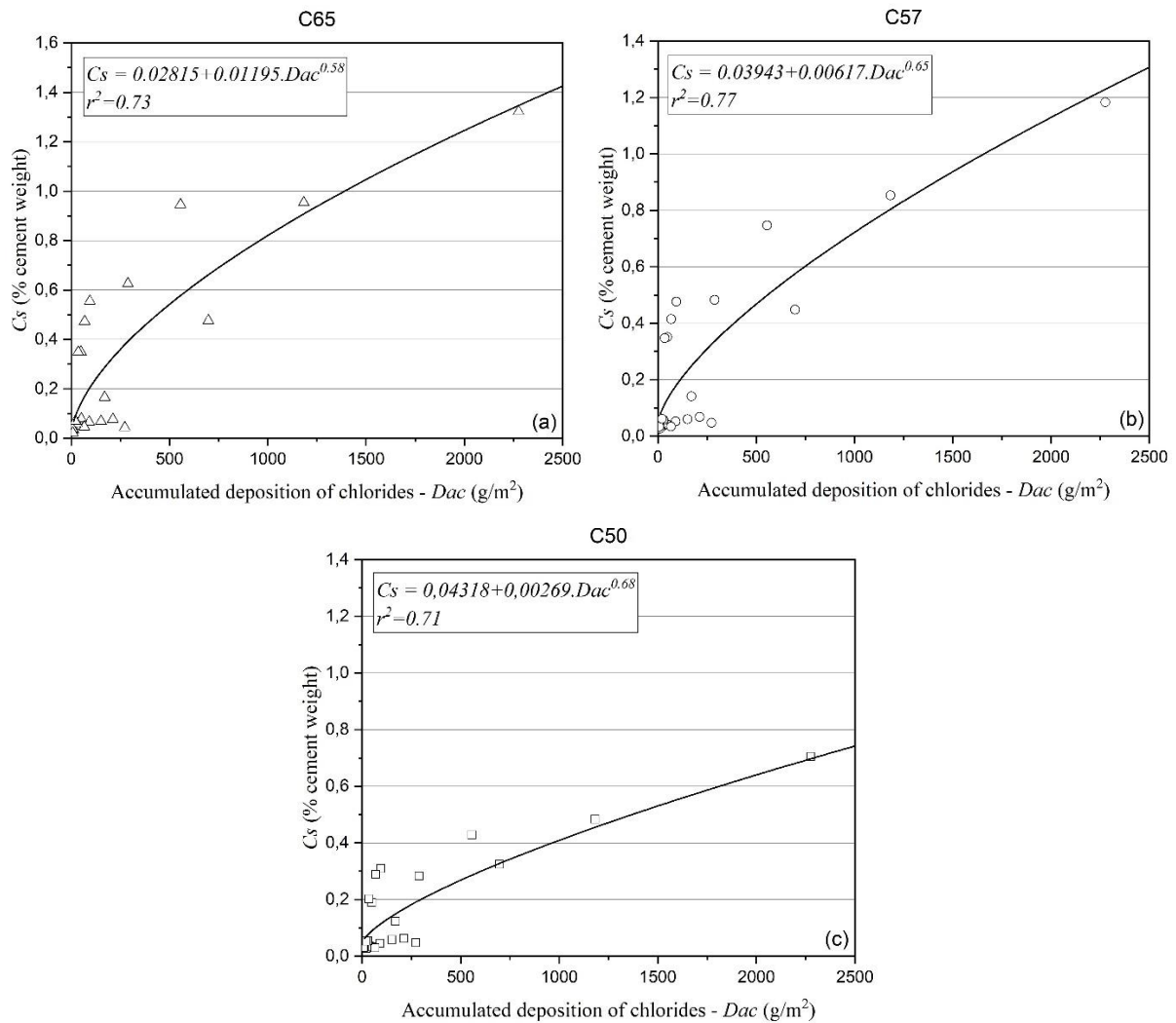


Figure 4. Relationship between C_s and accumulated deposition of chlorides (Dac) for concretes C65 (a), C57 (b) and C50 (c).

As can be seen in Figure 4, the relationship between C_s and Dac can be represented by the Equation (1), where C_s is the surface chloride concentration, C_0 is the initial chloride concentration in concrete, k_{cs} is a coefficient associated to the concrete ability in capturing

chlorides from atmosphere, Dac is the accumulated deposition of chlorides and n is a coefficient associated to the rate of Dac increase along time.

$$Cs = C_0 + k_{cs} \cdot (Dac)^n \quad (1)$$

This Equation follows a similar way of some previous studies when considered the influence of time on Cs increase (Uji *et al.*, 1990; Costa and Appleton, 1999; Zhou *et al.*, 2016). However, it has the advantage of taking into account the direct relation between the chlorides present in atmosphere and those captured in concrete surface.

4 Conclusions

The main conclusion that can be drawn up from the present analysis are:

- Cs increases along time and although tends to reach a maximum, this moment was not reached in 12.5 years of exposure time.
- Concrete porosity influences on Cs behaviour in a direct way. Concretes with higher w/b present a faster increase of Cs values.
- The availability of chlorides in atmosphere plays an important role in Cs behaviour. Higher availability of chlorides means higher Cs values. However, this relationship does not follow a linear function.
- The best function to represent the relationship between Cs and the availability of chlorides in atmosphere is $Cs = C_0 + k_{cs} \cdot (Dac)^n$, where Cs is the surface chloride concentration, C_0 is the initial chloride concentration in concrete, k_{cs} is a coefficient associated to the concrete ability in capturing chlorides from atmosphere, Dac is the accumulated deposition of chlorides and n is a coefficient associated to the rate of Dac increase along time.

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